Professional paper UDC: 622.332:621.331.22(497.6 Ugljevik) DOI: 10.7251/afts.2013.0509.015D

LABORATORY TESTING OF SAMPLES AT THE LOCATION OF THE THERMAL POWER PLANT - UGLJEVIK 3

Đuric Neđo¹, Đuran Perica¹, Miljanović Jovo¹

¹Technical Institute Bijeljina, E-mail: <u>tehnicki@tehnicki-institut.com</u> ²Faculty of mining Prijedor, E.mail: <u>miljanovic.jovo@gmail.com</u>

ABSTRACT

Field research for the purpose of the construction of the Thermal Power Plant Ugljevik 3 wasconducted in two phases. In the first phase for the smaller scope of the fieldworks, more laboratory analyses were performed. In the second phase, fieldworks were conducted in a larger scaleand laboratory analyses were considerably smaller.Designers dictated different volume of laboratory tests. In the first phase local legislation was respected while in the second phase Chinese planners considered that there was no need for larger volume of laboratory tests thereby giving more importance to the field study of drilling cores, SPT experiments and chemical analyses of the soil.

Key words: soil and rock samples, laboratory testing, geological environment

INTRODUCTION

Geological and geotechnical studies at the location of the Thermal Power Plant Ugljevik 3 were conducted in two phases - the first stage for the purpose of conceptual design and the second one for detailed design. Within the first phase of the research, a smaller volume of exploratory wells was carried out with accompanying laboratory tests. In the second phase significantly lower number of sampleswas prepared compared to the number of exploratory wells.

Depending on the characteristics of the rocks in a vertical profile, to a depth of about 30.0 m, disturbed and undisturbed soil and rock samples were taken. Samples were processed in the laboratory of the Technical Institute in Bijeljina.

Various volumes of sampling in the first and the second phase of the researchwere dependent on designers. In the first stage of the research, samples were taken in accordance with our local legislative on data requirements for sufficient exploration of geological environment. Designers who came from Chinese school that minimize significance laboratory tests in comparison to the fieldworks, especially SPR experiments and geophysics, were replaced in the second phase.

Sufficient volume of data from the first phase with data from the second phase anyhow provided basic knowledge of geological environment, that is about layers in a vertical section. This paper will present overall volume of laboratory analyses and methods applied as well as parameters for isolated areas.

SAMPLES OVERVIEW

During the exploratory drilling and preparation of exploratory wells, more samples were taken than it was delivered into the laboratory. Some samples were damaged during the removal or packagingand transport to the laboratory. Also, when selecting samples in the field and sending them to the laboratory, a selection is made if multiple samples from the same geological environment occur. After sampling has been completed selected sample iscovered with paraffin depending on whether it wastaken into cylinder or the cores, and transported to the laboratory on a daily basis.

Selection of samples for the laboratory undertakes the following:

- A description of the sample with certain data on:
 - o number of the well
 - depth of sampling
 - lithological description
 - o date of shipment to the laboratory

During the first phase of the research, from 35 wells, in the laboratory were taken:

- 194 samples, out of which:
 - 42 disturbed and undisturbed samples of the soil
 - \circ 152 samples of the rocks

In the second phase field research is considerably wider. It has been made 100 wells wherefrom significantly fewer samples were taken compared to the scope of the research:

- 130 samples, out of which:
 - 13 disturbed and 98 undisturbed soil samples
 - o 16 rock samples
- 3 samples of water

After sampling has been completed selected sample is covered with paraffin depending on whether it was taken into cylinder or the cores, and transported to the laboratory on a daily basis.

RESULTS OF LABORATORY ANALYSES

Laboratory testing of samples wereperformed in order toenable a proper way to receive all the samples that came in the laboratory of a specific institute, including the following:

- A list of all samples with specified data on:
 - \circ number of the well
 - depth of sampling
 - lithological description
 - date of receipt of the samples in the laboratory
- Selection of the samples by type of
 - the soil disturbed and undisturbed
 - o rocks
 - o water
- Selection of the samples according to the schedule of laboratory tests
- Conditional storage of all samples, including proper storage area where there will be no violation of the basic characteristics of the samples such as natural humidity, structure and shape.

In the first stage of the research, out of samples taken from 35 wells, in the laboratory were processed:

- 183 samples, out of which:
 - 34 disturbed and undisturbed soil samples
 - 149 rock samples

In the second stage of the field research, smaller number of samples was processed in the laboratory:

- 121 sample, out of which
 - o 10 disturbed and 95 undisturbed soil samples
 - 16 rock samples
- 3 samples of water

All the samples of the first phase were processed in the laboratory of the Technical Institute in Bijeljina while the samples of the second stage were analyzed in three laboratories:

- Laboratory of the Technical Institute in Bijeljina 103 samples of soil and rocks
- MOL from Belgrade 18 samples of soil for chemical analyses
- The Institute for Water from Bijeljina 3 samples

Samples selected for laboratory analyses preserved almost all of their natural features from the place and time of their sampling until thearrival in the laboratory.

On the basis of previously made programme of laboratory analyses, they were conducted in accordance with applicable standards in Bosnia and Herzegovina. The sequence of testing of samples in the laboratory of the Technical Institute in Bijeljina is shown in Figure 1.

In situ investigations defined geological environments in vertical profiles, and laboratory analyses determined their geomechanical properties.

Geological environment 1, belongs to Quaternary sediments of alluvial sediments and deluvial – proluvial formations. A complex of alluvial sediments is made of gravels, sands and clays, of thickness up to 4,0 m. Deluvial - proluvial sediments consist of clays and rarely clayey debris. They do not spread over the entire site and have maximum thickness up to 7.0 m. Complex of these sediments is characterized by plastic environments that behave plastically and rarely as a liquid.

Geological environment 2, belongs to a complex of rock substrates that is built ofmarly sediments such as marly clay, clay marl and clay marl with interbeds of marly clays with thickness from mm to dm. Transitional sediments from substrates towards Quaternary sediments are sediments of substrate weathering crustand sandstone blocks immersed into clayey ground. Substrate weathering crust sediments are not widely distributed while sandstone blocks immersed into clayey mass can be of larger dimensions. They have no larger distribution.

Geological environment 3, is built of marls with interbeds of sandstonesandof thickness from mm up to dm.Frequent alterations are presentand they are manifestedin certain intervals in the form of bright banded sandstones and dark bands ofmarls.In addition, the environment is characterized by the presence of marls alternating with clayey marl. Part of the marly rock complex is characterized by more expressed fractures, where the cracks are smooth and compressed. Softer marly rocks are typical of quasi-plastic medium.

Geological environment 4, is built of marly rocks with a small presence of sandstone. There can be found marls with interbeds and lenses of sandstone, sandy marl and marl with sandstone, which often alternates with clayey marl of thickness from cm to dm. Harder rocks are characterized by the presence of fractures usually at an angle of 45° while greater presence of softer rocks givesquasiplastic properties to the environment.



Figure 1.Sequence diagram of testing of samples in the laboratory of the Technical Institute

Geological environment 5, is made of sandstone that is characterized by alternations of sandstones and interbeds of marl, then sandstones and interbeds of marl and clayey marl. At the location it extends at a depth of 26.0 m. Rocks belonging to this region are very cracked and fractures are mostly rough and filled with clayey matter. Certain parts, containing mostly sandstone, are characterized by brittle areas and in the places of local character, quasi-plastic medium can be separated.

Laboratory tests were conducted in accordance with following applicable standards in the field of testing of soil, rock and water, Table 1.

Laboratory tests on soil samples					
TYPE OF TEST	STANDARDS				
Grain-size distribution	BAS CEN ISO/TS 17892-4:2009				
Consistencylimits- Atterberg limits	BAS CEN ISO/TS 17892-12:2009				
Specific gravity of soil	BAS CEN ISO/TS 17892-3:2009				
Volumetric weight of soil	BAS CEN ISO/TS 17892-2:2009				
Soil humidity	BAS CEN ISO/TS 17892-1:2009				
Water permeability ratio	BAS CEN ISO/TS 17892-11:2009				
Direct shear test	BAS CEN ISO/TS 17892-10:2009				
Soil compressibility test	BAS CEN ISO/TS 17892-5:2009				
Optimum water content – Modified Proctor's experiment	BAS EN 13286-2:2011				
Determination of California bearing ratio – CBR test	JUS U.B1.042				
Chemical analyses of the soil					
Sulphates, SO ₄ ²⁻	Standard method 4110 B				
Chlorides, Cl ⁻	Standard method 4110 B				
Calcium dissolved in water, Ca ²⁺	SRPS H .Z.181				
Magnesiumdissolved in water, Mg ²⁺	SRPS H .Z.181				
Bicarbonates, HCO ₃	Internal method				
Carbonates CO ₃ ²⁻	Internal method				
Gypsum	Internal method				
Total dissolved salts	Standard method 2540 B				
рН	EPA M 9045 D				
Laboratory to	esting of rock samples				
Ratio of density and porosity	BAS EN 1936:2009				
Uniaxial compressive strength	JUS B.B7.126				
Laboratory testing of water samples					
Total alkalinity	BAS EN ISO 9963-1:2000				
Carbonates	Calculated by the method BAS EN ISO 9963-1:2000				
Bicarbonates	Calculated by the method BAS EN ISO 9963-1:2000				
Sum Ca and Mg	BAS ISO 6059:2000				
$NH_3 - N$	BAS EN ISO 7150-1:2002				
NO ₃ . N	BAS EN ISO 10304-1:2002				
Chlorides	BAS EN ISO 10304-1:2002				
Sulphates	BAS EN ISO 10304-1:2002				
Total of solids	Standard methods 2540 (C)APHA-AWWA-WEF 2005				
Total of soluble matter	Standard methods 2540 (C)APHA-AWWA-WEF 2005				
Free CO ₂	Calculated by the method BAS EN ISO 9963-1:2000				
Calcium ₂₊	Standard methods 3500 (B)APHA-AWWA-WEF 2005				
Magnesium ₂₊	Computationally				

Table 1. Types and methods of laboratory testing of soil, rock and water

Results of laboratory analyses for selected geological environments are presented in Table 2, which shows the limit values from minimum to maximum [1, 2, 3, 4, 5]. These parameters cannot be used for geostatic analyses since they refer to the entire site where the research was conducted.

COMMENT ON THE RESULTS

Soil samples refer to geological environment 1 and rock samples to other geological medium. Due to the presence of faults in the research area, depths of individual mediums are different. In addition, there are frequent alternations of mediums in the vertical profile.

Values of parameters for all environments are moving in a slightly wider range and data that can be reliably used for geostatic analyses, necessary for design of buildings, are obtained by their correlation and translation into parameters of rock mass.

Facilities located in the complex of future thermal power plant are of different importance, both in size and content of the plants that require completely stable structure to be installed on. For these reasons, all necessary parameters were analyzed as well as conditions prevailing in geological environments, especially in the area of present faults.

Chemical analyses of the soil related to geological environment 1 showed the presence of various concentrations of individual elements. Concentrations are generally increased due to the presence of the Thermal power plant Ugljevik 1. Most of the particles of certain elements are transferred to the soil to a certain depth by means of surface water that infiltrates into the soil.

Various concentrations of particular elements in water samples are characterized by their movement through different geological environments at various depths affected by the fault area. In some places there is a direct connection of surface water with the fault zone, that is water from geological environment 1 interferes with water from the fault zone. In other parts of the field, the fault zone intersects individual geological environments in greater depth, aggravating the connection with surface waters.

The presence of the Thermal power plant Ugljevik 1 provides increased concentrations of certain particles in the air and on the ground and surface waters. To which extent are particles reflected into the surface water and further ontransferred towards underground water should be the subject of special research.

For a more detailed study of various concentrations of certain elements in underground water it is necessary to establish an observation network of piezometers and determine the sampling frequency.Filter construction should be installed on piezometers, in the media which the underground water is coming from and being affected by the faults.

(Received 24. july 2013, accepted 20. september 2013)

Đurić, N et al: Laboratory testing of

Archives of Technical Sciences 2013, 9(1) 15-23

Tabel 2. Results of laboratory testing of soil, ground and water

Laboratory tests	REZULTS						
	Geological environment	Geological environment	Geological environment	Geological environment	-		
	1	2	3	4	5		
SOIL	1	2	3	4	5		
Grain-size distribution							
gravel	0,00 %						
sand	7,15 - 38,45 %						
dust	41,79 - 74,65 %						
clay	10,59 – 16,55 %						
Consistency limits – Atterbeg limits							
Yield point W _L	28,91 - 51,33 %						
Plastic limit Wp	13,36 - 21,20 %						
Plasticity index Ip	15,58 - 34,40 %						
Consistency index c	0,582 - 1,323						
Liquidity Index I _L	0,113 - 0,373						
Specific gravity	$25,61 - 27,06 \text{ kN/m}^3$						
Volumetric weight of soil	$17,25 - 20,35 \text{ kN/m}^3$						
Porosity	30,94 - 40,62 %						
Soil humidity	9,68 - 24,87 %						
Water permeability ratio	$1,16x10^{-6}$ -7,06x10 ⁻⁹ cm/s						
Direct shear test							
Cohesion (c) CD and R	c=14-28 / 12-25 kN/m ²						
Angle of internal friction (ϕ) CD and R	φ=10-20 / 9-18 ⁰						
Soil compressibility test							
Modulus of compressibility Mv ₁₀₀₋₂₀₀	1721 – 10363 kN/m ²						
Modulus of elasticity $E_{200-100}$	44444 – 12654 kN/m ²						
Porosity ratio $e_{100-200/200-100}$	0,412-0,599/0,552-0,363						
Coefficient of consolidation $Cv_{100-200}$	$2,47 \times 10^{-3}$ - $7,21 \times 10^{-4} \text{ cm}^2/\text{s}$						
Compressibility index Cc	0,104 - 0,202						
Water permeability ratio	1,23x10 ⁻¹⁰ -7,37x10 ⁻¹¹ m/s						
Optimum water content – Modified							
Proctor's experiment	12,22 – 16,37 %						
Determination of California bearing							
ratio – CBR test	3,3-4,4 %						

*Technical Institute Bijeljina, Archives of Technical Sciences. Year V – N*⁰ 9.

Đurić, N et al: Laboratory testing of

Archives of Technical Sciences 2013, 9(1) 15-23

	1	2	3	4	5
Chemical analyses of the soil					
Sulphates, SO ₄ ²⁻	7,85 – 134,15 mg/kg				
Chlorides, Cl	21,29 – 128,89 mg/kg				
Calcium dissolved in water, Ca ²⁺	28,60 – 67,50 mg/kg				
Magnesium dissolved in water, Mg ²⁺	21,10 – 32,20 mg/kg				
Bicarbonates, HCO ₃	160,30 – 288,40 mg/kg				
Carbonates CO ₃ ²⁻	< 0,5 mg/kg				
Gypsum	< 0,5 mg/kg				
Total dissolved salts	1360,0 – 5360,0 mg/kg				
pH	6,04 - 7,47				
ROCKS					
Density		$2346 - 2482 \text{ kg/m}^3$	$2354 - 2558 \text{ kg/m}^3$	2491 – 2568 kg/m ³	$2498 - 2651 \text{ kg/m}^3$
Porosity		0,875 - 2,541 %	0,269 - 2,148 %	0,463 – 1,283 %	0,165 - 0,743 %
Humidity		1,11 - 8,65 %	1,72 – 5,79 %	0,71 – 5,42 %	0,64 - 3,84 %
Monoaxial strength		2,39 – 6,94 MPa	2,49 – 16,82 MPa	4,68 – 14,96 MPa	12,02 – 49,55 MPa
Angle of internal friction		13,13 - 25,07	13,80 - 31,88	16,49 - 33,56	27,27 - 40,25
Cohesion		0,44 – 1,48 MPa	0,84 – 1,92 MPa	1,28 – 3,42 MPa	1,65 – 6,37 MPa
Poisson's ratio		0.111 - 0,306	0,115 - 0,309	0,109 - 0,271	0,112-0,345
Modulus of deformation		148 – 778 MPa	140 – 1319 MPa	292 – 1494 MPa	1316 – 10478 MPa
Youngmodulus of elasticity		210 – 798 MPa	269 – 1369 MPa	398 – 1976 MPa	1447 – 13032 MPa
WATER					
Total alkalinity					$202,0 - 1054,0 \text{ g/m}^3$
Carbonates		Faultzone – passing through geological environments 2,3,4,5			$< 10,0 - 24,1 \text{ g/m}^3$
Bicarbonates					$248,0-665,5 \text{ g/m}^3$
Sum Ca and Mg				$28,0-536,0 \text{ g/m}^3$	
$NH_3 - N$		\bigcirc		$0,07 - 0,75 \text{ g/m}^3$	
$NO_3 - N$				$\leq 0.02 - 1.58 \text{ g/m}^3$	
Chlorides			27,74 – 51,55 g/m ³		
Sulphates			$0,56 - 109,2 \text{ g/m}^3$		
Total of solids			$500,0 - 1427,0 \text{ g/m}^3$		\mathbf{k}
Total of soluble matter		421,0 – 1178 g/m ³			
Free CO ₂		217,0-928,0 g/m ³			
Calcium ₂₊		8,7 – 155,8 g/m ³			
Magnesium ₂₊		$1,6-35,9 \text{ g/m}^3$			

LITERATURE

- Djuric, N. et al. (2012). Geotechnical investigation for Thermal Power plant 2 x 300 MW Ugljevik 3 in Ugljevik.Conceptual design level. Book I - text, Book II – graphic appendices, Book III – documentation of laboratory tests. Bijeljina.Archives of specific texts and documents
- [2] Djuric, N. et al. (2012). Geotechnical investigation at the site soil Planned Thermal Power plant Ugljevik 3 in Ugljevik. The level of the main project. Book I - text, Book II - graphic appendices, Book III documentation of laboratory tests. Bijeljina. Archives of specific texts and documents
- [3] Maksimovic, M. M. (2001). Soil Mechanics, second edition.Belgrade. Cigojaprint
- [4] Najdanovic, N., Obradovic, R. (1981). Soil mechanicsin engineering practice. Belgrade. Mining Institute
- [5] Maksimovic, M. M., Santrac, P. (2010). A collection ofitems from the Basicsof Soil Mechanics.6threvised edition.Belgrade. AGMbooks
- [6] Djuric, N. (2011). Hydrogeologicalandengineeringgeological exploration. Subotica, Bijeljina. Faculty of Civil Engineering, Technical Institute
- Barton, N. (1981). Suggested Methods for the Quantitative Description of Discontinuities. Int. Rock Mech. Sci., Vol 15.N⁰ 6 Pergamon Press Ltd.
- [8] Duncan, N. (1979). Engineering Geology and Rock Mechanics. London. Vol.I and II.Leonard Hill
- [9] Djukic, D. (2004). Geotechnicalclassification for surfaceworksin miningand construction. Tuzla. Mining Institute

Technical Institute Bijeljina, Archives of Technical Sciences. Year $V - N^{\underline{0}}$ 9.